

WET TECHNIK PBL REPORT

24.10.2019

Wet Technik College of Engineering, Design, Art and Technology Makerere University Kampala, Uganda

SUPPORTED BY OFFICIAL DEVELOPMENT AID FROM THE MINISTRY FOR FOREIGN AFFAIRS OF FINLAND

Appreciation

We would like to start by thanking the Almighty God for availing us with this opportunity to participate in the Problem Based Learning Project.

In the same way we would like to appreciate the Finnish government through the ministry of foreign affairs for supporting the Problem Based Learning Project together with Makerere University.

Special thanks to the PBL program coordinator Matleena Muhonen and the entire Aalto staff that participated in the program directly with Makerere University especially during the field trip.

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We would also like to appreciate Makerere University through the College of Engineering Design Art and Technology for offering us support in form of working space for laboratory analysis and a site to set up the pilot plant.

Lastly, I would like to appreciate fellow student participants from Makerere University in the Problem Based Learning project the entire Wet Technik team and the Vepox filter team. The Aalto student team for making the time we shared together during the field trip worthy by sharing knowledge and working together in building the projects we have now.

The Africa Hall kitchen staff for the hospitality and support offered to us during the construction of our pilot plant.



Abbreviations.

CEDAT- College of Engineering, Design, Art and Technology.

- PBL- Problem Based Learning.
- SGT- Sustainable Global Technologies
- MUK- Makerere University Kampala.
- CW- Constructed Wetland.
- ABR- Anaerobic Baffled Reactor.
- BOD- Biochemical Oxygen Demand.
- COD- Chemical Oxygen Demand.





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Project brief.

Makerere University, together with the University of Dar-es-Salaam, University of Nairobi and Aalto University in Finland, teamed up on a project on "Strengthening Problem- Based Education in East African Universities 2017-2020".

Problem-Based Education/ Problem-Based Learning (PBL) is a particularly powerful multidisciplinary approach to seeking and applying in-depth knowledge on particular phenomena prevailing in the society.

Makerere University Problem Based Learning project is run at College of Engineering Design Art and Technology and students in their Third Year of study in the various programs offered in the School of Engineering (SOE) and School of the Built Environment, Fourth year in Architecture; and Second Year in the Margaret Trowell School of Industrial & Fine Art participate in this project.

One team of students (4-6 members), with representation from all the three schools of CEDAT, is selected. The selected team is paired up with a team of students from Aalto University, to work on the identified project challenge. Students participating in the project are expected to gain hands-on experience in conceptualizing and prototyping environmentally, socially and economically sustainable solutions to community challenges. In addition, students will gain the knowledge tools of design thinking, project management skills, and join a regional and global network of PBL practitioners.

The project is funded by the HEI ICI Program by the Ministry for Foreign Affairs of Finland and facilitated by Aalto Global Impact, a home to Aalto University's societal impact initiatives.

The team participating in PBL is assisted to identify a mentor and team of experts from the three schools of CEDAT. The team works together with a complementary team of Aalto University students, from the fields of Engineering, Design, and Business. The student teams then work on prototyping their solution in the real world and test it.

Wet Technik is a team of students working under the Problem Based Learning (PBL) program at the College of Engineering, Design, and Technology at Makerere University. Wet Technik is looking at recycling wastewater through the use of constructed wetlands using novel materials such as plastic bottle caps and pumice.



Wet Technik is exploring the potential of grey-water recycling through the use of novel materials including pumice a locally sourced rock and plastic. The group is using constructed wetlands with improved aesthetics to harness the potential of greywater. The aim is to achieve water that can be reused in non-potable activities.

Wet Technik is constructing a pilot plant to study and treat the kitchen wastewater (greywater) from the Africa Hall kitchen and explore its potential for reuse in non-potable activities like irrigation.

Project objectives.

The project had a number of objectives and some of these include;

- To construct a pilot greywater treatment plant at Africa Hall.
- o To adopt the PBL approach to solving problems in the peri urban communities.
- o To develop networks with stakeholders both in the private and government sector.

Specific objectives.

- To carry out research on wastewater treatment systems and thereafter produce a full design.
- To engage stakeholders through workshops and site visits.
- To attend the 2-week field visit with Aalto University students and engage in all related activities.
- To carry out tests and further research on the pilot constructed wetland.



OUR TEAM



MUSINGUZI MARK MUSIIMENTA BSC. MECHANICAL ENGINEERING. YEAR IV.



SSEKIMPI DENNIS BSC. CIVIL ENGINEERING YEAR IV



KWIZERA PIKE

BSC. MECHANICAL ENGINEERING

YEAR IV.



BUZABO MELLISSA

BACHELORS OF INDUSTRIAL AND FINE ART.

YEAR III.





MWEBESA NINA SHATSI BSC. QUANTITY SURVEYING YEAR IV



MUSUMBA EMMANUEL BSC. MECHANICAL ENGINEERING

YEAR IV.





STAKEHOLDERS.



UNIVERSITY

- Provision of working space and laboratory for research

- Provision of access to site for construction of pilot plant.

-Mentorship and knowledge on circular design principles.



WEGE PRIZE

-Knowledge on business model generation



-Mentorship on design tailored for innovations to have a better climatic impact.



-Access to funding and mentors from UC Berkeley to develop startups solving critical issues.



-Access to mentors, business development advice and funding for early stage startups.



PROJECT TIMELINE

Laboratory tests



Preliminary Research and works done.

State of the Art analysis

Greywater is all wastewater that is discharged from a house, excluding blackwater (toilet water). This includes water from showers, bathtubs, sinks, kitchen, dishwashers, laundry tubs, and washing machines (fbr, 2005).

Greywater reuse is one of the ways to efficiently utilize water. There is need to identify cheap and easy to maintain systems that can make greywater available for reuse in the various settings i.e. household and institutions.

National Environmental Engineering Research Institute (NEERI) Nagpur and UNICEF Bhopal, Madhya Pradesh have developed, implemented and evaluated greywater reuse systems for small buildings (schools) in rural areas. During 2005 and 2006, NEERI AND UNICEF collaborated to investigate the possibility of recycling greywater (bathroom water) in residential tribal schools in rural Western Madhya Pradesh.

The drive for this technology was a result of decreasing availability of water, lowering of groundwater table and an increase in fluoride concentration in groundwater. It was discovered that a recycling system for about 400 students generating 4500l/day of greywater costs about \$1100 and the cost could be recovered in two years (Institute, 2007)

The reuse of greywater serves two fundamental purposes namely; reduction in the water requirements and reduction in sewage generation.

Greywater Treatment:

The treatment of greywater to avail it for reuse is considered basing on the reuse purpose, cost requirements and available area. The processes considered for treatment include anaerobic and aerobic processes.



General terms used to describe different degrees of treatment, in order of increasing treatment level, are preliminary, primary, secondary, and tertiary systems.

Preliminary treatment: The first phase of treatment in a series of treatment steps and typically it's intended to reduce the core solids content in wastewater before other treatment phases. It is basically made up of screens and grit removal mechanisms. This stage removes the materials that can cause operational problems.

Primary system: The major objective of this system is the removal of settleable organic and inorganic solids. The dissolved constituents in the wastewater are not affected in this stage of treatment. The

The primary treatment system is in itself a unit process with both physical and chemical processes taking place resulting not only in a reduction in solids (physical) but also other parameters like the Biochemical Oxygen Demand (BOD). The primary treatment system mainly consists of sedimentation tanks of different shapes i.e. round and rectangular.

Secondary system: This is majorly intended to achieve further treatment of the effluent from the primary system. Residual organics and suspended solids are the major pollutants removed in this stage. The biodegradable organic matter is also removed through various processes both aerobic and anaerobic.

Legislation Standards

Wastewater generation and discharge into the environment are governed by the National Environment Management Authority through the NEMA effluent discharge standards. The standards clearly stipulate based on various parameters the safe levels for the discharge of wastewater into the environment. Below is a summary of some of the parameters extracted from The National Environment Standards for Discharge of Effluent into Water or on Land.



Parameter	Standard
Biochemical Oxygen Demand	50mg/l
Chemical Oxygen Demand	100mg/l
рН	6-8
Turbidity	300NTU
Total Suspended Solids	100mg/l
Total Phosphorous	10mg/l

Table 1: Selected Permissible NEMA Effluent Discharge Standards

Source: The National Environment (Standards for Discharge of Effluent into Water or on Land) Regulations, S.I. No 5/1999

Based on the reuse purpose of the water effluent from a treatment plant can be compared to various standards. These include the potable water standards and water reuse standards.

Water to be used for potable purposes is governed by the Uganda Standard Potable water specification (US EAS 12). A summary of selected parameters form Uganda potable water standards is shown below:

Table 2: Potable Water standards

Parameter	Standard Treated Potable water
рН	6.5-8.5
Conductivity	1500(μS/cm)
Turbidity	5 NTU
Total Suspended Solids	Not detectable
Total Dissolved Solids	700mg/l
Total Phosphorous	10mg/l
Total Nitrogen(Nitrate)	45mg/l



The other reuse purposes like agriculture and other non-potable purposes are stipulated in various reuse standards especially those developed by international agencies. These include:

Table 3: International Reuse Standards

Organization	Guideline
World Health Organization (WHO)	Guidelines for the safe use of wastewater, excreta, and greywater (2006)
United Nations Environment Program (UNEP)	Guidelines for municipal wastewater reuse in the Mediterranean region (2005)
United Nations Water Decade Program on Capacity Development (UNW-DPC)	Proceedings of the UN-Water project "Safe use of wastewater in agriculture" (2013)
International Organization for Standardization (ISO)	ISO/TC282 Water reuse
Food and Agriculture Organization (FAO)	Water quality for agriculture (1994)

Primary System: Anaerobic Baffled Reactor.

The primary system is primarily concerned with suspended solids removal and along with this there is a change in the chemical parameters like the Biochemical Oxygen Demand (BOD). For the grey water management plant and Anaerobic Baffled Reactor was chosen as the primary treatment system.

Literature Review

Anaerobic Baffled Reactors (ABRs) are concrete, masonry or prefabricated fiberglass tanks consisting of several compartments in series (Tayler, 2018). They use anaerobic digestion in addition to settling of the particulate matter to remove organic material from the wastewater. They take advantage of the intimate contact between the wastewater and the active biomass that leads to bioremediation and the eventual further improvement in effluent quality is provided by relatively high retention times ranging from 24 hours to 72 hours.



Treatment performances of ABRs ranged from 37-80% COD reduction while BOD reduction lies between 60-90% reductions. (Pillay, 2004). The basic mechanical design of the ABR is very simple, and in situ installations need not have any parts outside the rectangular body. (Pillay, 2004).



Figure 1: Schematic of an Anaerobic Baffled Reactor

Source: (Tayler, 2018)

Key components:

Settler compartment:

This settler compartment separates large solids before progressing to the up-flow compartments. It reduces the rate of solids accumulation in the up-flow compartments and attenuates the peak flows so reduces the hydraulic load fluctuations on subsequent up-flow compartments. (Tayler, 2018)

Up flow compartment:

These compartments ranging from 2-8, are those in which the major treatment on the wastewater takes place.

Anaerobic filter:

As wastewater flows through the filter, particles are trapped and organic matter is degraded by the active biomass that is attached to the surface of the filter material. It is operated in up-flow mode because there is less risk that the fixed biomass will be washed out and solids are easily deposited at the bottom resulting in an effluent with less suspended solids.



Principle of operation

Pipes or baffles direct the wastewater from one compartment to another. The wastewater passes through a layer of settled sludge and provides intensive contact between organic pollutants and active biomass. Anaerobic digestion that takes place in an ABR consists of different groups of organisms. The four main steps that usually determine the organisms' reaction in an anaerobic process are: hydrolysis, acidogenesis, acetogenesis, and methanogenesis. (Tayler, 2018)

Although the anaerobic process is efficient in the removal of organic material and suspended solids from low strength wastewater, the process has no effect on nitrogen and phosphorus concentrations. In addition, pathogenic organisms within the wastewater are only partially removed. Post-treatment is therefore needed in removing residual COD and total suspended solids (TSS) as well as reducing concentrations of nutrients and pathogens.

After an anaerobic pretreatment, most often an aerobic post treatment is needed to meet effluent standards. This is where a tertiary treatment system that can efficiently polish the water such as a constructed wetland can be used.

Design of Anaerobic Baffled Reactor

Design parameters for the ABR system.

Up-flow velocity: This is the velocity of a fluid in a specified direction across a given cross sectional area. This is the area comprised of the base and the width of the ABR.

Hydraulic retention time: This is the resident time of wastewater in the entire system.

Compartment dimensions i.e. Length and depth of compartments: these are the dimensions of the up-flow compartments.

Design procedure for an ABR system:

The design for our ABR is broken down into the following major steps;

- Settler compartment sizing; Dimension the tank and design based on the recommended retention time.
- ABR up-flow compartment sizing; Dimension based on the required up-flow velocity



These steps are later broken down into the following;

Determine the loading on the plant i.e. Key parameters to be considered include BOD, COD and TSS.

Determine the peak flow rate.

$$q = \frac{Q}{t}$$

where; q- Peak flow rate

Q- Total flow rate

Determine the ABR compartment width.

$$w = \frac{q}{L v}$$

where; L- Baffled reactor compartment length.

v- Peak up-flow velocity.

Determine the retention time.

$$\theta = \frac{24 \, NcNsV}{Q}$$

where; Ns- Number of treatment streams

Nc- Number of up-flow compartments in series



Parameter	Symbol	Standards	Value	Unit
Peak daily flow rate	Qp	-	1.056	m³/d
Time of flow	Тр	-	12	h/d
Mean COD Influent Concentration	COD	-	1980	mg/l
Mean TSS Influent Concentration	TSS	-	956	mg/l
Depth of ABR reactor	Zc	1.8-2.5	1.5	m
Hydraulic retention time	q	48-72	48	h
Maximum up-flow velocity	n	1	0.25	m/h
Number of compartments	N(c)	4-8	3	
Organic loading rate	1	6		kg/m³d
UP-FLOW COMPARTMENTS				
Design parameters				
Peak flow			0.2	m3/d
ABR Compartment Width	W		1.3	m
Length of compartment	L		0.8	m
Number of treatment streams	Ns		0.4	
Volume of single compartment	V(p)		3.4	m3
Retention time	q		36.0	h
Organic Loading Rate	I		1.3	
SETTLER TANK		Units		
Recommended Detention Time	28.7	hrs		
Corrected Detention Time (Sludge Zone)	43.0	h		
Depth	1.5	m		
Width	1.3	m		
Length	2.7	m		
Length to width ratio	2.1			

Table 4: ABR dimension and parameter computation



Check for the organic loading rate.

 $\gamma = \frac{COD Q}{1,000 NsNcV}$ (Tayler, 2018)

V- Volume of single compartment (m³) = $\frac{wzL}{Ns}$ with; z-compartment depth, w-compartment width.

Settler compartment	Length	2.7m
	Width	1.3m
	Height	1.5m
Up flow compartments	Number	3
	Length	0.8m
	Width	1.3m
	Height	1.5m

Table 5: ABR Final dimensions

The drawings to illustrate the design adopted from the above calculations are as shown in the appendix.

Operation and maintenance of the plant

Start-up strategy

Optimum working conditions for the ABR system rely on the growth of the active microbial mass. This usually takes between 3-9 months and a high load of inorganic matter in the wastewater leads to the degradation of the existing microbial mass extending the start-up phase even further. (Tayler, 2018) With our COD: BOD ratio as 2:1, there is a considerably high amount of inorganic matter mainly from the soaps and detergents used in the various activities. E.g. washing of saucepans. This will affect the ABR systems performance especially in terms of COD reduction.



Recommended start up methods for the ABR system after construction dictate the need to start with comparatively high detention times of about 80 hours and reduce over a 30-day period towards the normal operating conditions of about 48 hours detention time. (Stuckey, 2010)

The maximum initial organic loading is 2kg-COD m³/d, and this cannot be exceeded during the start-up phase.

Vents will have to be constructed to safely release the methane produced during the anaerobic process in the compartments.

Maintenance:

Access points in the form of manhole covers will have to be created at the top of the compartments to aid in maintenance works mainly desludging and carry out sampling.

Secondary System: Constructed Wetland

Literature Review

The secondary treatment system consists of a constructed wetland which is a shallow basin filled with some sort of filter material (substrate), usually sand or gravel, and planted with vegetation tolerant of saturated conditions. (HABITAT, 2008)

The term constructed wetlands generally denotes all wastewater treatment facilities in which natural plants have a specific role. The name "constructed wetland for wastewater treatment" is in fact translation of the German term Pflanzenkläranlage, as these plants were initially conceived in Germany. In English speaking countries the term "constructed wetlands" (or treatment wetlands) is most often used.

Constructed wetlands are most often used as the second wastewater purification stage, i.e. in most cases, before actually reaching the constructed wetland body, the wastewater is subjected to preliminary and/or primary treatment. Various biological and physical processes such as adsorption, filtration, precipitation, nitrification, decomposition, etc. take place during operation of the constructed wetland. Many types of constructed wetlands can be used for treatment of various types of wastewater. They can thus be used for many purposes including:



- o Treatment or purification of municipal wastewater
- Treatment or purification of wastewater generated by individual households
- Subsequent (tertiary) treatment of waste water purified at conventional water treatment plants
- Treatment of some technological wastewater including seepage water from waste disposal sites
- Wastewater from oil refineries, or wastewater generated during agricultural production.
- Evacuation and mineralization of sludge separated from the waste water purification process
- Treatment and temporary retention of rain water.

Configuration of the wetland:

Constructed wetlands have various design configurations and they are as follows;

- o Life form of the dominating macrophytes (free-floating, emergent, submerged),
- Flow pattern in the wetland systems (free water surface flow; subsurface flow: horizontal and vertical),
- Type of configurations of the wetland cells (hybrid systems, one-stage, multistage systems),
- Type of wastewater to be treated,
- o Treatment level of wastewater (primary, secondary or tertiary),
- Type of pre-treatment,
- o Influent and effluent structures,
- Type of substrate (gravel, soil, sand, etc.), and
- Type of loading (continuous or intermittent loading). (HABITAT, 2008)

Nevertheless, the flow pattern has been generally accepted as the basic criterion. According to this criterion, constructed wetlands are classified into two basic types:

- Free water surface (FWS) constructed wetlands.
- Subsurface flow (SF) constructed wetlands.

Subsurface flow treatment wetlands are subdivided into Horizontal Flow (HF) and Vertical Flow (VF) wetlands depending on the direction of water flow.



Classification according to direction of flow:

Horizontal Flow (HF):

It is called HF wetland because the wastewater is fed in at the inlet and flow slowly through the porous substrate under the surface of the bed in a more or less horizontal path until it reaches the outlet zone. Due to the limited oxygen transfer inside the wetland, the removal of nutrients (especially nitrogen) is limited, however, HF wetlands remove the nitrates in the wastewater. (HABITAT, 2008)





(Source UN HABITAT)

Vertical Flow:

VF constructed wetland comprises a flat bed of sand/gravel topped with sand/gravel and vegetation. Wastewater is fed from the top and then gradually percolates down through the bed and is collected by a drainage network at the base. (HABITAT, 2008)





Figure 3: Illustration of a vertical flow constructed wetland

(Source UN HABTAT)

Components of Treatment Wetlands

Plant Vegetation:

Different plants are planted in the different sections of the wetland. Presence of macrophytes is one of the important features of a subsurface constructed wetlands. The plants transport oxygen to the root zone to allow the roots to survive in anaerobic conditions. The roots maintain the hydraulic conductivity of the substrate (sand, gravel, pumice, HDPE).

Role of plants in the treatment process:

The macrophytes have several properties in relation to the treatment process as well as aesthetic qualities that make them an essential component of the design. The plants used in subsurface flow constructed wetlands specifically designed for grey wastewater treatment should:

- Be able to tolerate high organic and nutrient loadings.
- Have rich below ground organs (roots and rhizomes) in order to provide substrate for attached bacteria and oxygenation.
- Be tolerant of saturated conditions.



Role of Plant tissue in water:

- Filtering effect
- o Reduced risk of re-suspension
- Increased rate of sedimentation
- Uptake of nutrients
- Increased aerobic degradation
- Surface for periphyton attachment



The most recommended plants to be used in subsurface flow constructed wetlands are emerged plants like cattail and common reed as well as robust plants like umbrella sedge, cyperus haspens, and broad leaved cattail. Some species of grass which are native to the tropical grasslands like elephant grass are also used. Decorative plants can also be used like bamboo plants, Heliconia, Canna lily, Calla lily, rhoeo strike me pink and papyrus sedges.

Bed Type	Root Depth (ft)	Final Effluent Quality (mg/L)		
		BOD ₅	TSS	NH3
Bulrush, Scirpus	2.62	5	4	2
Reeds, Phragmites	1.97	22	8	5
Cattails, Typha	0.98	30	6	18
No Plants	0	36	6	22

Figure 4: Other plants and characteristics

Pollutant and Pathogen Removal Processes in Treatment Wetlands:

Treatment wetlands are complex wastewater treatment systems possessing a diverse set of pollutant and pathogen removal pathways. Unlike other conventional wastewater treatment systems in which removal processes are optimized by a series of separate unit operations designed for a specific purpose, multiple removal pathways simultaneously take place in one or two reactors (Dotro, et al., 2017).

The major pollutants in wastewater include organic matter both particulate and soluble, which is measured by both Chemical Oxygen Demand and Biochemical Oxygen Demand. Nitrogen, phosphorous and pathogens are other forms of pollutants in the wastewater. The pollutant removal pathways include the following;

Pollutant	Removal Processes
Organic material (measured as BOD)	Biological degradation, sedimentation, microbial uptake
Organic contaminants (e.g., pesticides)	Adsorption, volatilization, photolysis, and biotic/abiotic degradation
Suspended solids	Sedimentation, filtration

Table 6: Pollutant removal mechanisms



Nitrogen	Sedimentation, nitrification/denitrification, microbial uptake, volatilization
Phosphorous	Sedimentation, filtration, adsorption, plant and microbial uptake
Pathogens	Natural die-off, sedimentation, filtration, predation, UV degradation, adsorption
Heavy metals	Sedimentation, adsorption, plant uptake

Source: East African Manual of Constructed Wetlands

On site works done

Study Area/Location

Location is a major factor when considering the design of a wastewater treatment plant. The location is considered such that the connections to the source of water are not made too long since wastewater contains several materials that can easily clog sewers and inhibit flow. There is, therefore, need to have regulated distances for the flow lines.

The site is also considered based on whether enough wastewater is generated for treatment and can be availed for reuse. Areas with limited wastewater generation do not offer sufficient reason for construction of a wastewater plant.

The greywater management plant is located strategically behind the Africa Hall Kitchen. The kitchen is a major source of wastewater and in particular greywater which is generated from the kitchen activities that are carried out during the course of the day and night.

The site also has sufficient land which satisfies the land requirements for a wastewater plant. The topography of this site is also suited for construction activities since limited cut



and fill activities were required for the construction process. The topography map is attached in the appendix.

Flow and Flow Variations

Determination of greywater generation and the flow rate is the first requirement in the design of greywater collection, treatment, and reuse system. Reliable data on existing and projected flow rate must be available for the cost-effective greywater treatment system design.

There are various methods which can be used in the determination of the flow values and these include;

Direct Methods:

These involve actual measurement of the wastewater generated at the site and involve the use of the bucket method and water meter method.

The bucket method involves use of a container of known volume to estimate flows for a designated period at an outlet point of a greywater generation area.

The water meter method involves the use of a meter connected to the pipe where wastewater flows that measures wastewater generated for a period of time.

Indirect Methods:

These involve the use of empirical relations to generate the flow values of wastewater. The methods include use of water consumption values and the type of uses.

Water Consumption method: From the available water consumption data at a site, the grey wastewater is assumed to be 50-60% of the total water consumption.

Type of uses method: From literature and previous studies per capita greywater generation for a given activity is identified and used to estimate the greywater generated from the site.

Selection of Method:

The bucket method was used to estimate wastewater flow values. This was chosen since the outlet point of the site was easily accessible and measurements could be carried out here. The method also allows easy observation of flow variation during different time



intervals of the day which is an important consideration in design of a greywater management system.

Measurement of the Flow:

Using the bucket method, a 70 liter drum was placed at the outlet and wastewater flow values were measure for an interval of 30 minutes for a period of 11 hours from 7am to 6pm which were identified as the active working hours of the kitchen facility at Africa Hall. This activity was done over a period of 7 days to ascertain the flow variation during weekdays and weekends.

The estimation of flow was carried out during the semester when the kitchen is active since meals are prepared for students during this period.

Variation of Weekly Average Values of Flow 80.00 70.00 60.00 Flow (litres) 30.00 30.00 20.00 10.00 0.00 7:30-8 9:30-10 10:30-11 1:30-2 4:30-5 7-7:30 8-8:30 8:30-9 9-9:30 11:30-12 12:30-1 1-1:302-2:30 2:30-3 3-3:30 3:30-4 4-4:30 5-5:30 5:30-6 0-10:3011-11:30 12-12:30 Time

The raw data of the wastewater flow values are attached in the appendix and clearly shows the variation of wastewater generation during the day and over a period of a week.

Figure 5: Chart showing flow variation



Computation of Flow rate at kitchen:

From the appended values of flow, the maximum value recorded within the 30 minute intervals was **140 litres.** The maximum daily average recorded for the 30 minute intervals was **44 litres.**

Considering a day of 12 active hours within which the kitchen users generate wastewater fron the various activities carried out. Using the average value recorded within the 30 minute intervals of 44 litres in every 30 minutes.

This results into 88 litres generated per hour of the day form the kitchen- (44*60 minutes)/30minutes = 88 litres

For the active day of work at the kitchen; 12 hours*88litres/hour = 1056 liters/day of 12 hours.

This is equivalent to **1.056m³/day** of wastewater generated from the kitchen.

Therefore, discharge Q = $1.056m^3/day$.

Wastewater Quality Characterization:

Greywater characterization is a requirement for the proper design of a wastewater treatment system. The chemical and microbial quality of greywater depends on the source of the greywater.

A typical qualitative composition of greywater is shown below indicating the various sources of grey wastewater;

Water Source	Bacteria	Chlorine	Foam	Food Particles	Nitrate	Oil and Grease	Organic	Soaps	Suspende d Solids	Turbidity
Cloth washing			*		*	*		*	*	*
Bathing	*					*		*	*	*
Kitchen	*			*		*	*	*	*	*

Table 7: Qualitative Greywater composition



Source: (Institute, 2007)

The characterization of the wastewater from Africa Hall kitchen was done in consideration of the following parameters and the following results obtained;

Parameter	Unit	Result
Biochemical Oxygen Demand	mg/l	881
Chemical Oxygen Demand	mg/l	1980
рН	-	6.76
Conductivity	μS/cm	1167
Turbidity	FAU	485
Total Suspended Solids	mg/l	956
Total Dissolved Solids	mg/l	1834
Total Phosphorous	mg/l	8.664
Total Nitrogen (Nitrate)	mg/l	25.5

Table 8: Greywater Characterization for kitchen effluent

Why an ABR was needed?

The wastewater characteristics we obtained from Africa Hall Cafeteria wastewater showed need for a pretreatment system that could be able to significantly reduce the BOD concentration, and not a simple pre-treatment system.

It is at this point that with advice from our mentors, we came to a consensus that an Anaerobic Baffled Reactor was the right system that could achieve this.

This led to an increase in the costs as a new unplanned system was introduced.

Selection of Parameters:



The parameters that were selected for evaluation were based on the reuse purpose, others as fundamental parameters for characterizing wastewater. The parameters were also selected to evaluate and choose the most appropriate pre-treatment system.

-pH: This is the measure of how acidic or basic (alkaline) the water and is important since it affects the survival of certain organisms which are vital for the treatment process of wastewater.

-Biochemical Oxygen Demand: This is the measure of the presence of organics in wastewater. It is an important parameter in determining whether the wastewater is safe for discharge in the environment.

-Total Nitrate and Phosphorous: The two parameters indicate the nutrient levels in the wastewater which depend on the source of greywater.

Design Approaches of Constructed wetlands:

The constructed wetland size can be estimated basing on different approaches which include;

-Rule of thumb	-Loading charts
-Regression Equations	-P-k-C*
-Plug flow k-C*	

The above methods are all applicable to horizontal flow sub-surface wetlands and take into consideration various aspects which include hydraulic loading rate, non-ideal flow, background concentration and pollutant weathering.



Comparison of the design approaches:

Table 9: Advantages and disadvantages of design methods for constructed wetlands

Advantages	Disadvantages				
Rule of Thumb					
-It is very simple to use.	-It does not account for different water usage practices, pre-treatment technologies, climate, or influent wastewater concentrations.				
	-It does not account for non-ideal flow.				
	-It does not consider the geometry of the wetland cell or specific design approaches to minimize the risk of clogging.				
Regression Equations					
 They are simple to use. They take into account influent water quality. They inherently account for background concentration (C*) because equations were created from actual water quality data from full-scale systems. 	 They are only applicable if the design of the new wetland falls within the data range from which the regression equations were created. Many regression equations were created from very large treatment wetland systems, and may not apply to smaller systems. Flow rate is not always considered. The wetland area cannot be determined from equations that only correlate concentration or mass. 				



Plug-flow k-C*					
-It takes into account influent concentration (Ci), background concentration (C*), HLR (q) and areal reaction rate coefficient (kA).	-It does not account for non-ideal flow, which creates a large risk, especially when low effluent concentrations must be achieved.				
-lt can take into account temperature correction factor (θ).	-There is no guidance as to which kA- value to choose (for example, when a range of reaction rate coefficients are reported).				
Mass Loading Charts					
P-k-C* approach					
It accounts for influent and effluent concentration (Ci and Co), as well as background concentration (C*). It accounts for areal or volumetric	There are many variables to assess and many have only limited information from which to select appropriate design values for a specific condition.				
reaction rate coefficients (kA or kV) and temperature correction factors (θ). • The designer can choose the level of risk	The value of P depends on the geometry of the wetland unit, and its selection needs to take this into account.				
(50%, 80% or 90% compliance) for certain design variables.	The designer must be extremely familiar with all of the material provided in Kadlec and Wallace (2009) in order to understand and locate the required design information.				

Source: (Gabriela , et al., 2017)

From the above methods the P-k-C* approach was selected to establish the size of the wetland including area. The method considers areal and volumetric rate coefficients and temperature correction factors. The



P-k-C* illustrated:

$$\frac{(C-C^*)}{(C_i-C^*)} = \frac{1}{(1+\frac{k_A}{Pq})^P} = \frac{1}{(1+\frac{k_\nu\tau}{Pq})^P} \qquad \text{Kv}_{=} \text{ k} = \frac{k_A}{\varepsilon V/A} = \frac{k_A}{\varepsilon h}$$

Where;

0

C= outlet concentration, mg/L

Ci = inlet concentration, mg/L

K= first-order rate coefficient, 1/d

 τ = nominal (theoretical) hydraulic retention time, d

P = apparent number of tanks-in-series (TIS), dimensionless

q= hydraulic loading rate, m/d

 k_v = modified first order volumetric rate constant

 ε = porosity (fraction of wetland volume occupied by water), unit less

V = wetland volume, m^3

A= wetland surface area, m²

h =wetland water depth, m



Design Consideration:

-Influent and outlet concentrations: The target influent and effluent concentrations of the wastewater are considered and can be expresses using the various parameters BOD, TSS, COD TN and TP.

-Flow rates (Q): This is the value of the wastewater considered for the design of the wetland.

-The Hydrologic budget: Consideration for precipitation (P) events, evapotranspiration (ET) -which affect the flow rates considered for design of the wetland.

-Hydraulic Residence Time(HRT): The HRT is defined as the time a molecule of water stays in the wetland on average, from entrance to exit, and is typically calculated as the water volume in the reactor divided by the flow rate. (Gabriela , et al., 2017)

-Porosity: This is considered basing on the substrate material to be used in the wetland.

Design Procedure for Constructed Wetland

This is a design procedure that follows the P-K-C* approach according to Kadlec & Wallace (2009).

Procedure:

The procedure illustrated is for a design considering the pollutant concentration for Biochemical Oxygen Demand (BOD).

1.Setting influent flow and concentrations:

Considering a 50% reduction in BOD in the primary treatment system, the influent concentration into the wetland is 441mg/l

The flow considered is 88 litres per hour (2.93x10⁻⁴m³/s)

2.Setting target effluent concentrations

Derived from the Guidelines and Standards for Wastewater Reuse, an effluent BOD level of 30mg/l is appropriate for effluent to be used in back yard irrigation and toilet flushing.



-Selecting k- rates for targeted parameters

-Appropriate trend multiplier (1 + ψ) according to the regulatory compliance interval in which case it is the re-use standards

C= C_{trend} (1+ ψ): C_{trend} is the mean value of the desired effluent= 1.5 for an average effluent of 14mg/l; where C is the influent concentration.

Selection of P value (Apparent number of Tanks in Series)

It is important to consider variability in nature of flow through the wetland therefore the wetland is considered as a series of tanks.

P=3 for BOD in a horizontal sub-surface flow wetland.

3.Selection of C* values for BOD loading:

The background concentration (C*) is an irreducible effluent concentration that results from internal biogeochemical cycling within wetlands. (Gabriela , et al., 2017)

(Kadlec & Wallace, 2009) suggest computing background concentration from the equation below for a given BOD concentration.

$C^* = 0.6 + 0.4(Ci)^{0.55}$

4. Estimation of wetland area:

The area is obtained from the p-k-C* equation earlier stated and afterwards the mass loading checked against the loading charts if appropriate and adjustments to the k values can be considered until an appropriate area is obtained. The area chosen was based on the parameter that gives the largest area. The Biochemical Oxygen Demand gives the largest area requirement.





Figure 6: Summary of design procedure for constructed Source Kadlec & Wallace (2009)

Table 10: Parameters and Dimensions of constructed wetland:

Input Parameters	Value	Output Parameters	Value
		Outlet Pollutant	
		Concentration	
Inlet Pollutant Ci (mg/l)	441	(mg/l)	18.46897
		Hydraulic Retention	
Apparent TIS rate constant k (m/yr)	377.1	Time (days)	2.88
Apparent number of TIS for pollutant			
reduction(P)	3	Volume	20





Porosity	0.35(Gravel)		
	0.25(Pumice)	Effluent Mass	
	0.95(Plastic)	Loading gBOD/d	53.19064
Depth h	0.5		
	18(Gravel)		
	16(Pumice)		
Area (m²)	10(Plastic)		
Background Concentration C* (mg/l)	28		
Flow Rate (m3/d)	1.056		
Areal Rate Coefficient (ka) m/yr	66		

Considering the largest area generated from the substrate of the lowest porosity as the overall wetland area. The overall area is then divided into compartments for the different substrate. The goal is to assess the variation in performance of the different substrate placed in a wetland basin.

Substrate Configuration:

The wetland consists of pumice, plastic (waste bottle caps) and gravel. They are arranged in the following ways to enable experimentation of the various materials in terms of treatment efficiency.







Figure 7: Substrate configuration

Plant Configuration:

The presence of plants is one of the important features in subsurface flow constructed wetlands. They have several properties in relation to the treatment process as well as the aesthetic qualities that make them an essential component of the design.

ution	Horse tail reed	Sampling point	Horse tail reed	Sampling point	Horse tail reed
water distrib	Horse tail reed	Sampling point	Horse tail reed	Sampling point	Horse tail reed
Point of	Horse tail reed	Sampling point	Horse tail reed	Sampling point	Horse tail reed



Direction of water flow

Figure 8: Plant configuration

The selected plant was horsetail reed. It is an ever green plant that looks like a mini bamboo. Grows in sandy or gravel areas and can withstand water logged areas. A new seedling takes 20-45 days to grow and it grows up to 0.91m tall. It has a dense fibrous root system that is about 2m deep.

Operation and Maintenance



The wetland vegetation should be monitored to ensure that unwanted plant species (weeds) do not overtake the intended plant community (Gabriela , et al., 2017). This is achieved through removing the weeds.

Other major maintenance operations include;

-Adjustment of water levels

-Maintenance of flow uniformity (inlet and outlet structures)

-Management of vegetation

-Odor control.

FIELD VISIT

Field Visit to Kisoro.







Team Communication



Figure 9 - Makerere PBL Teams having a video call with the Aalto University Team



Internship activities

Setting Out

This is the transferring of points from paper (drawings) to ground through use of measuring techniques. It is done to enable realization of construction works according to the drawings.

Materials and Tools used:

Pegs: These are important in marking the positions identified during the setting out process.

Tape measure: This is helpful in measuring the required dimensions based on the drawings.

Mallet: This is used in entering the pegs into the ground during the setting out process.

Rope: This is used during setting out to demarcate straight edges.

Square: This tool is used for marking the right angle during the setting out procedure.

Procedure:

The setting out process was done using the square method where a builder's square is used to mark out the straight edges from corners. On the site, the setting out was done to guide the excavation works. The areas for construction of the anaerobic baffled reactor and constructed wetland were set out based on the drawings.

The reference point for the anaerobic baffled reactor (ABR) was the Africa Hall kitchen wall. At equal distances of 2m points were marked on the ground and this line was taken as the edge of the ABR. The constructed wetland reference line was considered as the slab of the gas cage and its edge was marked in reference to the line of this slab.

The points were marked using pegs and placed at distances indicated on the drawings to mark the first corners of the units to be constructed and a rope connected to these pegs.



Using a square and rope the right angle was identified and the distance to the next corners was marked with a tape. The point was marked with a peg and the procedure repeated for all the corners of the units.

The procedure was done in reference to the drawings and the site layout.





Figure 10: Setting out activities

Excavation works

The anaerobic baffled reactor and constructed wetland were constructed at certain depths below the ground. There was therefore need to excavate the set out areas.

Tools and equipment:

Hoes, peak axe, -Used for dislodging the earth material and creation of a pit.

Spades- Removal of earth material from the excavated pit.

Procedure

The set out area was excavated using hoes, peak axes and the earth material removed using spades. The wetland site was excavated to a depth of 0.5m and the Anaerobic Baffled Reactor site to a depth of 1m.



The earth was removed and heaped on the edges and left to be used for backfill purposes

The excavated base was levelled and the level was checked using a water level to ensure that the base was evenly levelled.



Figure 11: Excavation of site

Casting of concrete Blinding

A concrete mix of 1:2:4 (cement, sand, aggregate) was cast for the concrete blinding in both the excavated pits of the constructed wetland and the ABR. The concrete was laid to a depth of 100mm in the ABR pit and 50mm in the constructed wetland pit.

Tools and materials

Wheel barrow- Wheel barrow was used to move materials like cement from the store to the point where mixing of concrete would take place. It was also used to transport sand from the point where it was stacked to the point where mixing would take place. The wheel barrow was also used to transport concrete from the mixing point to the point where concrete would be cast.

Mortar Pan- This was used to hold mortar and use where the wheelbarrow would not be used comfortably especially while working in the pit.

Pegs or Marker-These were used as gauges to indicate the required elevation of the concrete. A marker would be used to indicate gauge on the formwork while a nail would be nailed into the formwork to also indicate a gauge.

Hose pipe- This was used for spraying water for curing onto the surface of concrete.



Straight edge- This was used to level the concrete surface after casting by moving it along the surface and ensuring no aggregates are exposed while leaving a smooth finish.

The materials included;

Cement- This was used as binder to bond the aggregates together. The cement which was used was kept in the store in-order to protect it from adverse weather conditions like rain that reduces the quality of the cement through un-wanted hydration reactions.

Fine aggregate- The fine aggregates where used as a filler to cement and lake sand was used for most cases.

Coarse aggregate- This was used in the concrete and aggregates of 0.5 inch were selected.

Procedure:

Fresh concrete was prepared from the mixing point on the site, transported to the point of placing (the excavated pits of the ABR and the constructed wetland).

In the pit a mesh and Damp proof material were placed across the entire area prior to casting the concrete. Pegs were driven into the ground at different points to help in monitoring the concrete gauge.

Using pegs driven into the ground at different point in the excavated pit to a required depth of the concrete to be cast, the concrete was placed to achieve the required gauge.

The concrete was spread evenly until the required gauge was achieved and using a straight edge the concrete was levelled.

The concrete was left to set and cured using water from a hose pipe before the construction activities proceeded.





Figure 12: Casting of blinding concrete

Brick work of compartment walls

The brick work was done to create the up flow compartments in the ABR and the cells in the constructed wetland. A mortar mix of 1:4 (cement, sand) was used for both the brick joints and the finishes.

Tools and materials

- Wheel barrow- Wheel barrow was used to move materials like cement from the store to the point where mixing of mortar would take place. It was also used to transport sand from the point where it was stacked to the point where mixing would take place. The wheel barrow was also used to transport mortar to the point where brick work would be taking place.
- Spade- It was used to load the wheelbarrow with fine aggregates and it was also used mix and mingle the mortar several times before and after water was added.



- Mortar Pan- This was used to hold mortar and use where the wheelbarrow would not be used comfortably.
- Trowel- A trowel was used to create apply mortar into the joints and also to apply mortar beds for the bricks. The trowel would also be used to create even surfaces of the mortar joints and mortar beds.
- Plumb bob- A plumb bob is an instrument that is used to check the verticality of any structure. In this case the plumb bob was used to check the verticality of walls and bricks relative to other set bricks.
- Spirit Level- A spirit level was used in checking the elevations of the block's top surface also to transfer level from one brick to another.
- Water level- Water level was an instrument used to transfer elevations of the bricks from one brick to another.
- Tape measure- This was used to make measurements of the positions of the bricks during the construction of the wall compartments.
- String-This was used to check for linearity of the wall itself and its relative alignment with other walls.

Procedure:

First the mortar was mixed manually by loading the sand onto the wheelbarrow and transferred to the mixing area to form a heap. The mixing area was a concrete platform to prevent the mortar from being contaminated with soil.

Water would then be added to the mix by carefully making a depression within the cement sand heap. Mixing of the mortar would then continue until the mortar becomes homogeneous and attains a uniform colour.

The mixed mortar would then be loaded onto the wheelbarrow and transferred to where the brick work was to be done.

The walls from at the very bottom were set first. The setting involved using a string placed against a reference point to adjust the initial bricks of the wall. Before setting the initial brick, some water is added to the surface of the blinding and this is done to ensure that the surface does not absorb water from the mortar which would cause it to crack.



After setting out the initial bricks, and a string is maintained in position, the subsequent bricks would be laid following the string to check for linearity of each and every brick. This was done until all the first course is done.

The subsequent courses would then be laid but every other brick on the second and subsequent courses would be checked for verticality using a plumb bob.

To ensure a uniform elevation throughout the full length of the wall, a brick at one end of the course is made the reference point and is used to transfer levels to the brick on the other end of the wall. In case the levels would not be the same, mortar adjustments would be done to ensure uniform elevations of the wall.

In case of a T-junction of the brick work. A builders' square would be used to make sure the walls are at right angles as indicated on the plan.

The walls of the ABR were raised to a height of 1.3m while those of the constructed wetland to a height of 0.8m. The process for both units involved fitting of 4inch pipes at different sections as openings for fittings which were placed later on.







Figure 13: Brick wall construction

Plastering and Finishes

Plastering was done to create a layer of mortar around walls, which provides dampness protection to the walls and also improves on aesthetics of the structure. Plastering was done using cement and sand mortar with a ratio of 1:4. It is important to note that the water cement ratio of mortar to be used for plastering was different and higher than the water to cement ratio of mortar to be used in block work.

Sikalite a water-proofing admixture was used in the mortar mix for every bag of cement used. The sikalite occupies the capillary spaces formed in the mortar mix and increases the water-proofing properties of the mix to protect the walls of the structure from moisture and prevent loss of water through seepage.

Tools and Materials:

Fine aggregate (sand) - The fine aggregates where used as a filler to cement so as to create mortar.

Cement- This was used as binder to bond the fine aggregates and to improve the adhesion of mortar on the walls and columns. The cement which was used was kept in the store in-order to protect it from adverse weather conditions like rain that reduces the quality of the cement through un-wanted hydration reactions.

Water- This was used to mix the mortar and also to wet the surface before the initial bricks would be set out. The importance of water was to react with cement and form a binding medium.



The tools included;

Wheel barrow. Wheel barrow was used to move materials like cement from the store to the point where mixing of mortar would take place. It was also used to transport sand from the point where it was stacked to the point where mixing would take place. The wheel barrow was also used to transport mortar from the point where mixing was done to the plastering area. It was used to load the wheelbarrow with fine aggregates.

Mortar Pan- This was used to hold mortar where the wheelbarrow would not be used comfortably.

Trowel. A trowel was used to apply mortar on to the surface of the wall.

Wooden float. This was used to fill the gaps left when the mortar is cut and to generally apply a smooth finish.

Steel float. This was used to apply lime to the plastered surface and create a cement screed finish.

Straight edge. It was used to level the plastered mortar to form even surfaces.

Procedure:

The mortar was mixed using the procedures discussed under brick work activities. Mixing of mortar was done using manual means.

Setting up of gauges to indicate the exact thickness of mortar required on the wall. This was done by applying mortar thickness to the wall. The required thickness was then measured off from the wall and marked and the gauge was then transferred to all the other points on the wall.

Application of cement sand mortar onto the wall followed after and it was done using a trowel.

The mortar was levelled using an aluminum straight edge. This was then followed by application of a wooden float.

A cement screed finish was applied to the plastered wall using a steel float. A steel float helps to give a smooth finish.





Figure 14: Finishing works

Casting of the Anaerobic Baffled Reactor Slab

A concrete slab of thickness 150mm was placed above the walls of the ABR and it was placed with voids to create space for access chambers. The reinforced concrete was finished and left to set while curing it with water from a hose pipe to gain strength enough for support of loads.

Formwork/shuttering

Formwork was installed to be able to contain fresh concrete until it acquires a set that enables it to withstand loads. Formwork was done for the ABR slab. Wooden formwork



was applied since it is cost effective and the timber can be used for other applications after it is removed.

Timber pieces of different dimensions were used, i.e. 4"x2", 12"x1"and eucalyptus poles used as props. The tools and equipment that were used to make formwork included;

Bow saw-This tool was used to cut timber pieces to the required dimensions. It consists of a blade and a handle.

Craw hammer- The craw hammer was used to fix timber pieces together to form joints and connections. The craw hammer was used to hit nails in the timber members.

Nails- The nails were used to join the different timber pieces.

Tape measure- This was used to measure the dimensions of the different pieces prior to sawing.

The decking support of the slab was done using timber boards. The timber boards of dimensions $12^{"}x1^{"}$ were supported by $4^{"}x2^{"}$ runners that were then held by wooden props. The props were prepared to hold the $4^{"}x2^{"}$ pieces in position below the boards.

The props were placed at a spacing of 0.3m across the entire slab board. The slab board was surrounded by 12"x1" pieces placed according to the required gauge of concrete slab to be cast.

Reinforcement:

Steel works were done and involved fixing steel reinforcements for concrete works of the ABR slab and manhole covers. Steel reinforcements involved setting up re-bars for the slab. The bar sizes that were used included; T12 for the main bars and R8 for the manhole handle bars.

Tools and Equipment:

High tensile steel bars-These formed the reinforcement bars for the different structures of concrete.

Bar bender- This was a tool used to bend the high-tensile steel bars into hooks and required shapes. It was also used to straighten the steel bars where it was required.

Binding wire- The binding wire was used to join the steel bars together in the reinforcement cage.



Axel saw- This was used to cut the steel reinforcement bars

Tape measure-This was used to measure off the required length of the bars and the hooks.

Procedure:

The steel reinforcement bars were delivered to site in the folded form for ease transportation. They were then straightened on site manually using a bar bender.

A tape measure was used to measure the required length of the steel bar and the points to be cut were marked.

An axel saw was then used to cut the steel bars to the required length.

A bender was used to make the hooks and bends of the length specified to create the anchors.

A damp proof material (DPM) was placed at the base of the formwork to seal the gaps left within the slab formwork boards and prevent the water from the concrete from seeping through. The DPM also protects the concrete from having direct contact with the wooden formwork which does not leave a smooth finish after the concrete has set.

The steel bars were then positioned within the formwork in either direction at the spacing of 40mm as specified in the drawings.



Figure 15: Reinforcement cage on ABR slab



Concrete Casting:

- The freshly mixed concrete was then loaded onto a wheel barrow which moved it to the casting place for the slab.
- A mortar pan was then be used to pour the concrete into the formwork and this is done until the gauge mark was reached.
- The concrete was spread evenly until the required gauge was achieved and using a straight edge the concrete was levelled.
- The concrete was left to set and cured using water from a hose pipe as the construction activities proceeded.
- After a period of 14 days, the formwork was stripped off from the concrete and prepared for application of finishes. The slab was allocated enough time to set and gain strength before being loaded.



Figure 16: Casting of ABR top slab

Filling of substrate

The substrate used in the constructed wetland was done in reference to the substrate configuration illustrated in the constructed wetland design section. Gravel plastic in the form of waste plastic bottle caps and pumice were placed in the compartments as shown in the layout;







Substrate specifications:

The constructed wetland was filled with gravel of 0.5 inch diameter. The gravel was cleaned using water to remove fines that increase solid particles in the wastewater flowing through the wetland.

The pumice was packed in two forms i.e. randomly packed and linearly packed (Straight rows of piled pumice rocks)

The anaerobic baffled reactor in the anaerobic filter chamber was filled with gravel placed on a concrete support. The concrete support was perforated such that water rises through the substrate (up flow mechanism).





Figure 17: Substrate in Anaerobic filter and constructed wetland



RESULTS AND ACHIEVEMENTS.

Prizes:

WEGE PRIZE

The Wege Prize is an annual competition that ignites game-changing solutions to problems. It focuses on challenging multidisciplinary groups of students to redesign the way economies work through adopting circular economy principles. Wet Technik participated in this competition and was able to reach the finals where we emerged in third place.



CLIMATE LAUNCHPAD.

This is one of the largest green business idea competitions in the world that helps identify and develop startups that look to have a positive climate impact. We were



able to complete in 3rd place of the National Finals and we will compete at the Global Finals in Amsterdam in November.

Partner	Role				
United Social Ventures	Wet Technik has been able to				
	successfully partner with United Social				
VENTURES	Ventures a supporting agency for social				
	ventures creating a pipeline for youth-				
	led social start-ups in Uganda to				
	become more impactful, financially				
	sustainable, and scalable, through				
	different programs.				
Resilient Africa Network	This organisation enhances resilience-				
related knowledge and shares it glo					
ROM	engaging students, faculty, staff, and				
"· · · · · · · · · · · · · · · · · · ·	development experts from around the				
Stillions through innovation	world to collaborate on solving				
	resilience related problems.				

Established Networks and Partnerships:

Conclusion

The entire PBL experience has been one of the most exciting and educative processes that we have gone through. We are truly grateful for the opportunity to interact with mentors from all walks of life.

We have developed so many skills as a team and we have been able to appreciate the importance of a multidisciplinary approach to problem solving.



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Annex

	Time		Flow Value (Ltrs)								
DAY		Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday	Average	Max	Total
	7-7:30	52.5	70	70	140	70	70	0	67.50	140	473
	7:30-8	52.5	35	70	70	70	35	0	47.50	70	333
	8-8:30	52.5	70	35	65	52.5	105	8.75	55.54	105	389
	8:30-9	52.5	70	35	52.5	17.5	52.5	0	40.00	70	280
	9-9:30	52.5	17.5	17.5	52.5	17.5	17.5	0	25.00	52.5	175
	9:30-10	52.5	35	35	70	35	17.5	52.5	42.50	70	298
	10-10:30	35	70	17.5	52.5	10	17.5	52.5	36.43	70	255
	10:30-11	35	17.5	17.5	52.5	17.5	8.75	0	21.25	52.5	149
	11-11:30	35	70	35	52.5	10	17.5	17.5	33.93	70	238
	11:30-12	17.5	17.5	35	35	35	17.5	0	22.50	35	158
	12-12:30	17.5	35	17.5	17.5	5	17.5	35	20.71	35	145
	12:30-1	17.5	17.5	17.5	10	17.5	17.5	17.5	16.43	17.5	115
	1-1:30	35	15	35	0	52.5	17.5	17.5	24.64	52.5	173
	1:30-2	8.75	87.5	0	17.5	5	17.5	0	19.46	87.5	136
	2-2:30	8.75	20	0	0	17.5	52.5	17.5	16.61	52.5	116
	2:30-3	17.5	17.5	5	0	17.5	8.75	0	9.46	17.5	66
	3-3:30	0	35	35	35	0	0	0	15.00	35	105
	3:30-4	35	60	0	60	0	0	0	22.14	60	155
	4-4:30	17.5	52.5	17.5	52.5	17.5	0	17.5	25.00	52.5	175
	4:30-5	140	35	35	35	17.5	8.75	0	38.75	140	271
	5-5:30	17.5	70	35	70	17.5	0	0	30.00	70	210
	5:30-6	17.5	17.5	52.75	17.5	35	17.5	35	27.54	52.75	193
Average		35.00	42.50	28.08	43.52	24.43	23.47	12.33	27.14		
max		140	87.5	70	140	70	105	52.5	55.54		
Total		770	935	617.75	957.5	537.5	516.25	271.25			

Appendix 1: Daily flow values from Africa kitchen







